Strength and Elastic Properties of Low-Fine Self-Compacting Concretes Designed with Nano SiO₂

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Abstract - Self-compacting concrete (SCC) is increasingly takes place in construction applications due to its excellent self-compacting characteristic. It is known that, in order to achieve self-compacting feature, it is necessary to use high volume of fine materials in the mix design with effective superplasticizers. Using high volume of fine materials causes a decrease in the amount of total coarse aggregates. Coarse aggregates have an important role on the strength and elastic properties of soccentes. This paper presents the results of an experimental study which investigated the strength and elastic properties of SCC mixtures modified with nano-SiO₂. Nano-SiO₂, having 35nm average particle size, was used with the aim of reducing the total fine material in SCC designs. Five different nano-SiO₂ percentages (0.5%, 1.0%, 1.5%, 2.0% and 2.5%) were utilized and the total fly ash content used in the reference mixture was gradually reduced. The volumetric emptiness occurred by the fly ash reduction was filled by aggregates. Successful mixtures which exhibited desired SCC properties were subjected to compressive tests at 28th and 120th days. Moduli of elasticity of concrete specimens were also measured. Results have shown that with increasing W/Cm ratio, compressive strengths were decreased. The use of nano-SiO₂ in reduced-fly ash content mixtures could not compensate the strength decrement. However, the use of nano-SiO₂ combined with fly ash has prominently enhanced the elastic modulus of nano-modified SCC mixtures.

Keywords: Self-compacting concrete, Nano SiO₂, Strength properties, Modulus of elasticity

1. Introduction

Self-compacting concrete (SCC) is a new generation innovative material by its special properties. These properties such as consolidating under its own weight, filling formworks without any vibration or easier casting for dense reinforced members, makes SCC more popular in the last decades [1, 2]. The most general rules in order to obtain self-compacting ability are limiting the coarse aggregate content, lowering the water to binder ratio and using a powerful superplasticizer [3]. High flowability causes tendency of segregation and bleeding during transportation and placing. In order to prevent the risk of segregation and bleeding and to achieve necessary viscosity, it is recommended to increase the amount of fine materials or using viscosity modifying agents [4].

It is known that aggregates play an important role for designing high-performance concrete [5]. In order to improve the mechanical properties of the concrete, strength, brittleness, texture and mineralogy of the coarse aggregates are very important [6]. Mechanical properties of a hardened concrete are directly related to the mix design parameters [7]. For the safety, durability and serviceability of a concrete member, compressive strength, tensile strength and the modulus of elasticity are the most important parameters [8]. Many researchers reported that there is a direct relationship between aggregates and compressive strength of concrete [9 - 13]. Elastic modulus is also related to the stiffness and the characteristics of the phases that constitutes concrete [14]. Therefore elastic modulus directly affected by the aggregate content and some researches showed that aggregates have a very significant effect on elastic modulus [15, 16]. In the literature, it was reported that SCC has lower elastic modulus than conventional concrete since the lower aggregate content and the smaller maximum grain size [17 - 19]. Applications of nanotechnology in cement based systems have attracted much attention in recent years. Especially using nano particles in cement based systems has a huge potential [20]. Nano particles, which have enormous specific surface area, leads to extreme chemical reactions and could modify the nano-structure of hardened cement paste. A considerable amount of studies in the literature have reported that nano particles influence the mechanical properties of cement based systems [21 – 27].

In this study, SCC mixtures modified by nano-SiO2, on the purpose of reducing the total fine material amount has been investigated. In line with this purpose, fly ash which was used as fine material in the mixture designs was gradually reduced. The volumetric emptiness occurred by the reduction of fly ash was filled by aggregates without changing the grain distribution. In this way, total aggregate content was increased. On hardened state, compressive strength and modulus of elasticity of the concrete samples at 28th and 120th days were determined. Thereby, the influence of increased total aggregate content and the use of nano-SiO2 on the mechanical and elastic properties of nano-modified SCC mixtures were examined.

2. Materials

CEM I 42.5 type ordinary Portland cement used in the study. Fly ash was obtained from Catalagzi thermal power plant in Turkey. Chemical composition and some physical properties of cement and fly ash are given in Table 1. Four different aggregates were used in the study, which are 0-2.5 mm natural sand, 0-4 mm crushed sand, 4-11 mm (CS I) and 11-22 mm crushed stone (CS II). The densities of the aggregates were 2.63, 2.68, 2.69 and 2.69 g/cm3 respectively. Aggregates used in the study have a fineness modulus of 1.65, 2.49, 5.55 and 6.57 respectively. Cembinder 8 by AkzoNobel was used as colloidal nano silica (CNS) in the study. Properties of nano SiO2 is given in Table 2. The CNS has a solid content of 50% (50% SiO2 and 50% water by weight). All the replacement rations given in the study for CNS denote net nano SiO2 amounts. As super plasticizer, modified polycarboxylate ether polymer based high performance super plasticizer was used.

Composition, %:	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	CaO	MgO	SO ₃	Cl [.]	Density (g/cm ³)	Surface area (m²/g)
Cement	19.28	5.45	2.79	64.41	2.07	2.76	0.017	3.14	0.337
Fly Ash	58.75	25.24	5.76	1.46	2.22	0.08	0.015	2.00	0.222

Table 2: Properties of colloidal nano SiO₂.

Table 1: Chemical composition and physical properties of cement and fly ash.

Property	Colloidal Nano SiO ₂
SiO ₂ Content, wt%	50
Water Content, wt%	50
Average Particle Size, nm	35
Density, g/cm ³	1.4
Surface Area, m ² /g	80

3. Experimental Study

Table 3 shows the mixture designs of concretes showing desired SCC characteristics. CNS was added to SCC mixture by five different amounts which are 0.5%, 1.0%, 1.5%, 2.0% and 2.5% of total binder by weight. Besides for each CNS percentage, four different fly ash levels were used. In reference mixture 160 kg/m³ fly ash was used and this amount was reduced step by step down to 0 kg/m³. At every step, 40 kg/m³ of fly ash was reduced. Volumetric emptiness caused by the fly ash reduction was filled with the addition of aggregates without changing its grain distribution. Mix percentages of aggregates by volume are 22.0%, 19.0%, 29.5% and 29.5% for CS1, CS2, natural and crushed sand, respectively. In this way, the total aggregate volume fraction in SCC mixtures was increased from 59.3% (for reference mixture) to 66.3% (for 0FA mixtures). However, the change in coarse aggregate concentration in these mixtures is very limited, i.e. from 24.3% to 27.1%. Average aggregate volume fractions of the reference and FA mixtures are given in Table 4.

In order to understand that the mixtures were proper as an SCC, preliminary SCC flowability tests were performed such as T_{500} , D-mean, V-funnel. Some mixtures did not provide SCC properties due to high segregation, bleeding or the loss of consistency. These mixtures accepted as unsuccessful mixtures and denoted with star in Table 3. Unsuccessful mixtures were ignored in the mechanical tests. Mixtures were produced in a laboratory type mixer and cast into cylindrical molds with 100 mm diameter and 200 mm height. Samples were demoulded after 24 hours of setting and placed into lime saturated water cure tank. Samples were cured for 28 and 120 days. After curing period compressive strength tests were performed according to TS EN 12390-3 and static elastic moduli of samples were measured according to TS ISO 1920-10.

Mix ID	WIC	W/C-	Watan	Comont	E A	Nano	SP	CS I	CS II	Fine Aggregate	
MIX ID	W/C	W/Cm	Water	Cement	FA	SiO2 ^{**}	SP			Nt. Sand	Cr. Sand
Reference	0.46	0.33	185	400	160	0.0	7.84	351	303	460	469
FA120/S0*	-	0.36	185	400	120	0.0		362	313	475	484
FA120/S0.5						2.8		361	312	473	482
FA120/S1.0	0.46					5.6	7.84	360	311	472	481
FA120/S1.5	0.40					8.4	7.04	359	310	470	479
FA120/S2.0						11.2		358	309	469	478
FA120/S2.5						14.0		356	308	467	476
FA80/S0*			185	400	80	0.0	7.84	374	323	490	499
FA80/S0.5		0.39				2.8		373	322	488	498
FA80/S1.0	0.46					5.6		371	321	487	496
FA80/S1.5	0.46					8.4		370	320	485	495
FA80/S2.0						11.2		369	319	484	493
FA80/S2.5						14.0		368	318	482	491
FA40/S0*						0.0		385	333	505	515
FA40/S0.5*		0.42	185	400	40	2.8	7.84	384	332	504	513
FA40/S1.0	0.46					5.6		383	331	502	512
FA40/S1.5	0.46					8.4		382	330	500	510
FA40/S2.0						11.2		381	329	499	508
FA40/S2.5						14.0		379	328	497	507
FA0/S0*	0.46		185	400	0	0.0	7.94	397	343	520	530
FA0/S0.5*		0.46				2.8		396	342	519	528
FA0/S1.0*						5.6		394	341	517	527
FA0/S1.5						8.4	7.84	393	340	515	525
FA0/S2.0						11.2		392	339	514	524
FA0/S2.5						14.0		391	338	512	522

Table 3: Mixture designs of SCC mixtures (kg/m³).

W/C: Water to cement ratio, FA: Fly ash, SP: Superplasticizer, CS: Crushed stone, Nt. Sand: Natural sand, Cr. Sand: Crashed sand *Unsuccessful mixtures as SCC, **Pure nano silica amounts.

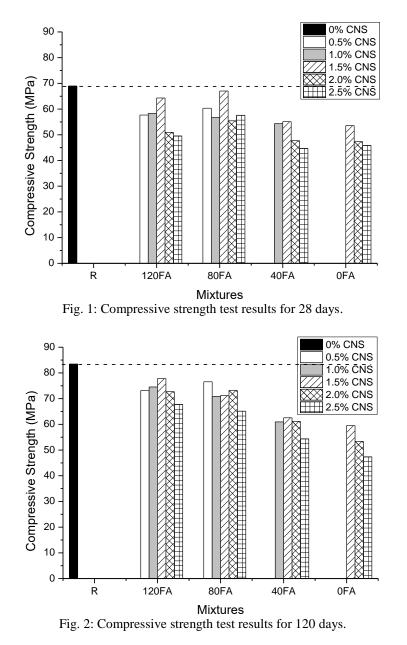
Table 4: Aggregate volume	fractions	ofSCC	mixturas	(l_{α}/m^3)
1 able 4. Aggregate volume	machons	01 SCC	minitures	(Kg/III).

	Aggregate Volume Fraction, %					
Mix Code	Coarse Aggr.	Fine Aggr.	Total			
Reference mixture	24.3	35.0	59.3			
FA120 mixtures	24.9	35.7	60.6			
FA80 mixtures	25.7	36.9	62.6			
FA40 mixtures	26.5	38.0	64.5			
FA0 mixtures	27.1	39.1	66.3			

4. Results

4.1. Compressive Strength Test Results

In order to specify the compressive strength of the concrete samples, compressive strength tests were performed on 28th and 120th days, according to TS EN 12390-3. The results of compressive strength tests are illustrated in Figures 1 and 2.



Results have shown that both on 28th and 120th days reference sample has given the highest compressive strength results. Compressive strength values of 68.9 MPa at 28th day and 83.4 MPa at 120th day were obtained from the reference mixture containing 160kg/m3 fly ash without nano-SiO2. As a general trend, with respect to reference SCC mixture, compressive strength of lower fly ash content mixtures with nano-SiO2 decreased with decreasing fly ash content. Although water to cement ratio (W/C) for all the mixtures is kept constant at 0.46, increasing water to total binder ratio (W/Cm) with the reduction of FA content, however, has resulted with the decrement in compressive strengths. It can be

concluded that the use of nano-SiO2 in reduced-fly ash content mixtures could not compensate the strength decrement. The closest value to reference sample on 28th day was obtained from the mixture 80FA/S1.5 (67.0 MPa). On 120th day, however, the mixture 120FA/S1.5 has shown the closest compressive strength value to reference mixture (77.8 MPa). Based on the compressive strength results shown on Figures 1 and 2 and only considering the mixtures with nano-SiO2, the optimum dosage of nano-SiO2 would be 1.5%.

4.2. Elastic Modulus Test Results

In order to understand the effect of adding nano-SiO₂ and increasing the total aggregate content on the elastic modulus of SCC samples, elastic modulus test under compression was performed according to TS ISO 1920-10. Elastic modulus test results are shown in Figures 3 and 4.

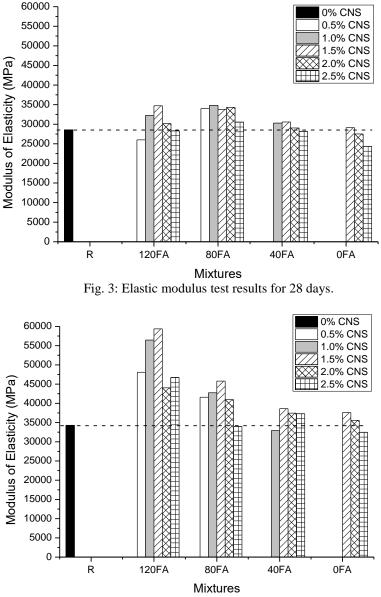


Fig. 4: Elastic modulus test results for 120 days.

On 28th day, a major part of the mixtures and on 120th day, almost all of the mixtures have higher elastic modulus results than reference mixture. The elastic modulus of reference samples was 28.6 GPa on 28th day and 34.3 GPa on 120th

day. For the 28 day cured mixtures, 80 kg/m3 fly ash containing samples has shown the highest elastic modulus values. Samples in this group have reached up to 22% higher values than reference sample. 40 kg/m3 and no-fly ash samples containing nano-SiO2 have lower elastic moduli than other sample groups but similar to reference mixture. Especially the results for 120th day samples containing nano-SiO2 is noteworthy. On 120th day, 120 kg/m3 fly ash containing mixtures have shown elastic modulus values "dramatically" greater than the reference mixture. Mixture 120FA/S1.5 has the highest elastic modulus value with 59.4 GPa and this value is 73% higher than the reference sample. 80 kg/m3 fly ash containing mixtures have maximum 33% higher value than the reference mixture. 40 kg/m3 and no-fly ash samples, on the other hand, have closer elastic modulus values to reference sample at 120th day. It should be reminded that, with respect to the reference mixture, the compressive strength values of low fly ash mixtures containing nano-SiO2 decreases with the reduction in fly ash content. However, contrary to expectations from the trend in compressive strength change, compared to reference mixture, the elastic modulus values are significantly increased especially for 120FA and 80FA mixtures. From the results obtained, it can be concluded that the increment in elastic modulus of low fly ash nano-SiO2 mixtures (compared to reference mixture) cannot be related to the increment in aggregate volume fractions of these mixtures. In fact, the aggregate volume fractions of the reference and 120FA mixtures are 59.3% and 60.6%, respectively, which are very similar. Moreover, W/Cm ratio of the reference mixture (0.33) is lower than the W/Cm ratio of 120FA/S1.5 mixture (0.36), and the compressive strength of the reference mixture (83.4 MPa) is slightly higher than the compressive strength of 120FA/S1.5 mixture (77.8 MPa). In order to explain this extraordinary elastic behavior, how the structure of C-S-H gel is effected by the use of nano-SiO2 and fly ash combination should be deeply studied. On the other hand, within the low fly ash nano-SiO2 mixture groups (120FA, 80FA, 40FA and 0FA), although the aggregate volume fractions increases with decreasing fly ash content (from 60.6% to 66.3%), the reduction observed in elastic modulus would be related to the reduction of compressive strengths in these groups.

5. Conclusions

Compressive strength and the modulus of elasticity values of samples produced with different amounts of CNS, which was used in order to reduce the total fine material amount and increase the total coarse aggregate content, were investigated. In accordance with the experimental results, following conclusions can be drawn.

- 1. With respect to reference SCC mixture, compressive strength of lower fly ash content mixtures with nano-SiO2 decreases with decreasing fly ash content.
- 2. Although water to cement ratio for all the mixtures is kept constant at 0.46, increasing water to total binder ratio with the reduction of FA content has resulted with the decrement in compressive strengths. The use of nano-SiO2 in reduced-fly ash content mixtures could not compensate the strength decrement.
- 3. Contrary to expectations from the trend in compressive strength change, compared to reference mixture, the elastic modulus values are significantly increased especially for 120FA and 80FA mixtures containing nano-SiO2.
- 4. Compared to reference mixture, the increment in elastic modulus of low fly ash nano-SiO2 mixtures could not be explained with the increment in aggregate volume fractions of these mixtures.
- 5. In order to explain the observed extraordinary elastic behavior of nano-SiO2 containing mixtures, how the use of nano-SiO2 and fly ash combination is effected the structure of C-S-H gel should be deeply studied.

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