

The Effect of Polypropylene and Steel Fibers on the Engineering Properties of Concrete

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Abstract - This paper presents results of an experimental investigation in which the effect of polypropylene, PP, and steel fibers, SF, on the mechanical properties of plain concrete was investigated. The effect of steel fibers only on the shear capacity of reinforced concrete beams was also studied. The results of the polypropylene and steel fibers were exerted with normal concrete; the studies show that the polypropylene reduces the mechanical properties of concrete whereas the steel fibers enhanced the mechanical properties of concrete as well as the shear capacity of the reinforced concrete beams.

Keywords: Mechanical properties, shear capacity, steel fibers, polypropylene, deflection

1. Introduction

An understanding of the effect of polypropylene, PP, and steel fibers, SF, on plain and reinforced concrete is necessary for design of various concrete structures. Polypropylene fiber is a high monofilament which inhibits the formation of plastic shrinkage and plastic settlement cracking by providing an internal support system for concrete. This takes place when the concrete is beginning to harden and shrink due to volume changes from loss of water, most structures using polypropylene hold up well to earthquakes and fire. Steel fibers, SF, which is made from hard drawn steel wire, improves the cracks resistance (or ductility) capacity of the concrete, it also bridging across smaller crack forming after the application of load on the member, therefore preventing their widening into larger cracks

2. Experimental Program

2.1 - Details of test Specimens

The research described in this paper consists of two different parts. The first part deals with the effect of polypropylene, PP, and steel fibers, SF, on the mechanical properties of concrete. The following tests were carried out to study the latter;

1. compressive strength on 150x150x150 mm³ cubes ^[1]
2. Indirect tensile splitting on 100x200 mm² cylinders ^[2]
3. flexural strength, modulus of rupture, MoR, on 100x100x500 mm³ prisms ^[3]
4. static modulus of elasticity on 150x300 mm² cylinders ^[4]
5. water absorption on 150x150x150 mm³ cubes ^[5]

The effect of steel fibers on the shear capacity of reinforced concrete beams was studied in the second part of this investigation. Eight reinforced concrete beams of cross section 80mm x130 mm and 1300 mm longer were cast with 12 mm diameter high tensile bars in the tension zone, the specimens were divided into 4 groups as shown below in Fig (1a-d) ^[6]

1. beam 1 : beam without links and poor anchorage of longitudinal reinforcement, figure 1a

2. beam 2 : beam without links and good anchorage of longitudinal reinforcement figure 1b
3. beam 3 : beam with links and poor anchorage of longitudinal reinforcement, figure 1c
4. beam 4 : beam with links and good anchorage of longitudinal reinforcement, figure 1d

The poor anchorage detail was straight anchorage of 40mm, i.e.3.5 times the bar diameter past the centre line of the support. 2.5 mm mild steel bars were used as shear reinforcement when links were provided. 6 mm diameter mild steel bars were provided as compression reinforcement in beams 3 and 4. One beam from each of the previous groups were cast from steel fiber concrete, this was compared into identical beams cast from normal concrete. The two groups of beams were tested in similar way.

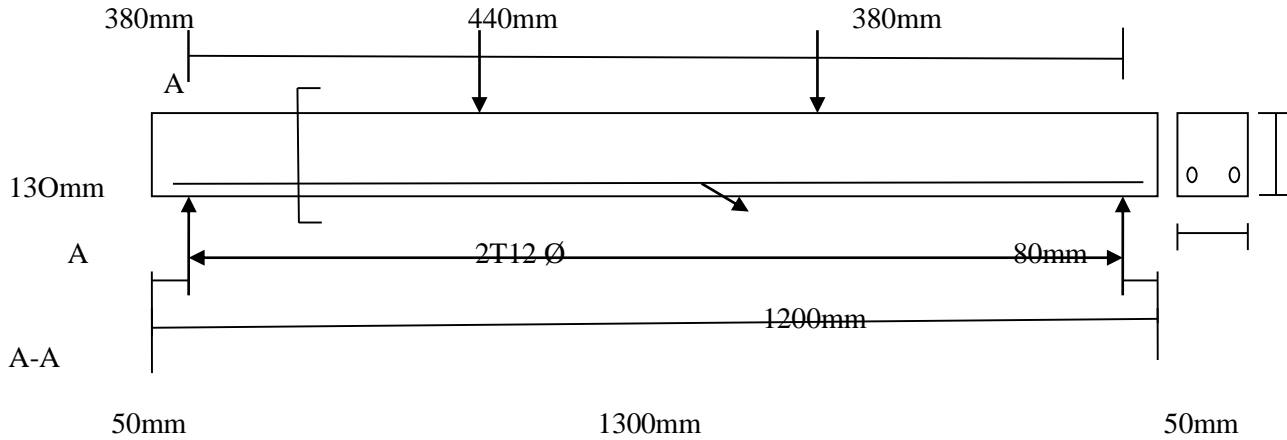


Fig. 1a: beam 1 - beam without links and poor anchorage of longitudinal reinforcement.

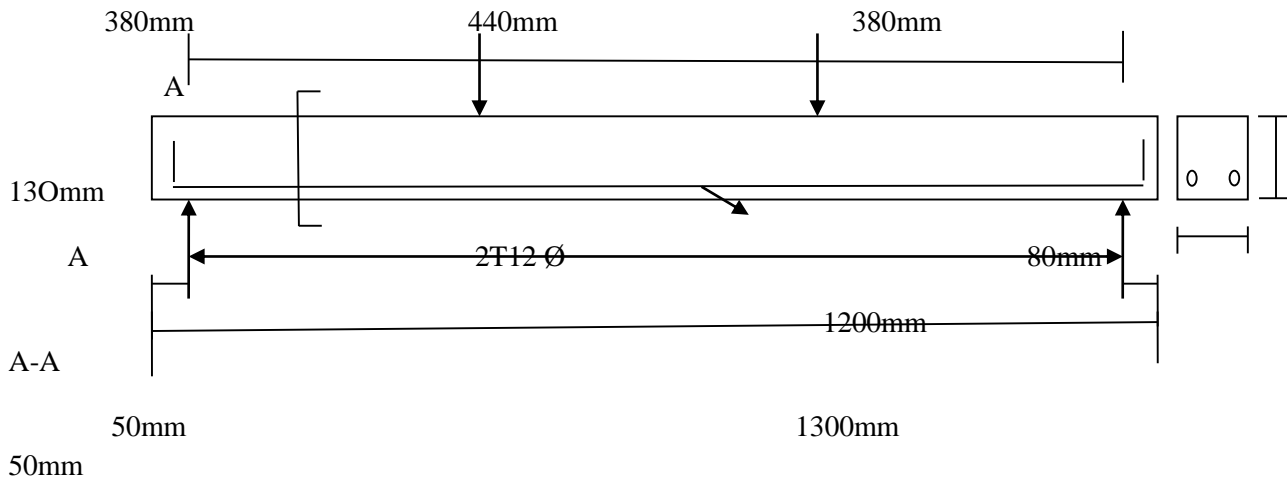


Fig. 1b: beam 2 - beam without links and good anchorage of longitudinal reinforcement.

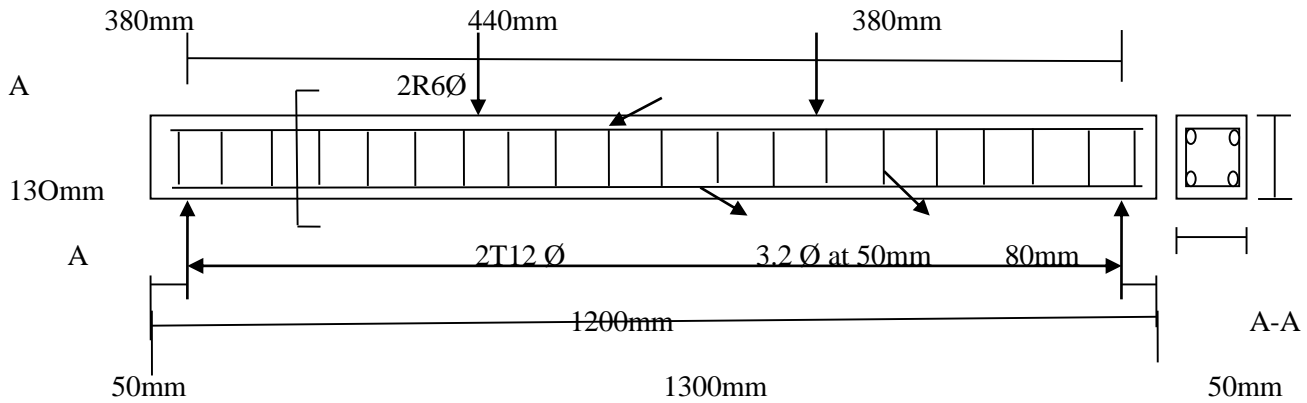


Fig. 1c: beam 3 - beam with links and poor anchorage of longitudinal reinforcement.

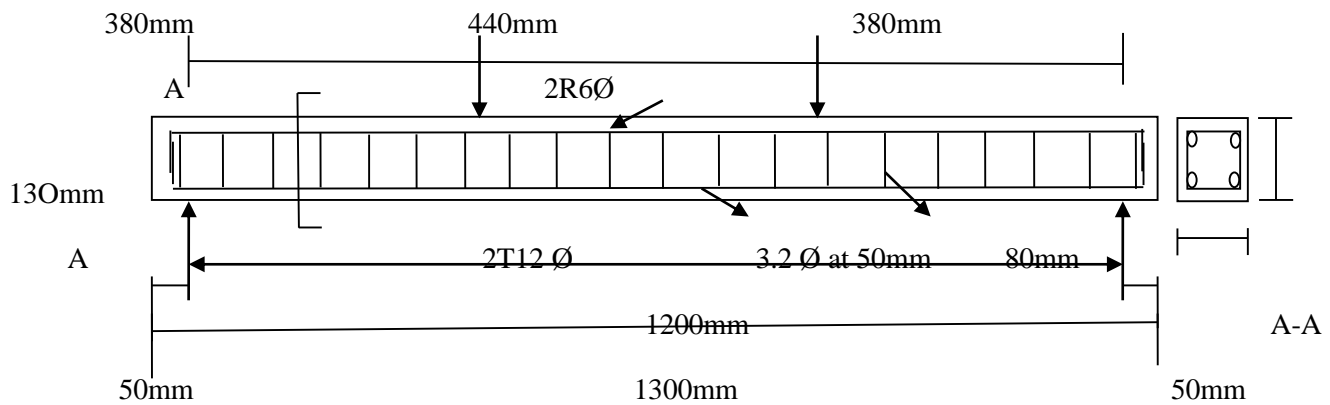


Fig. 1d: beam 4 - beam with links and good anchorage of longitudinal reinforcement.

3. Concrete Mixes And Material Properties

Four mixes were used to study the effect of polypropylene on the mechanical properties of plain concrete and another four mixes were used to investigate the effect of steel fiber on plain and reinforced concrete.

The contents of the polypropylene for mixes 1, 2, 3 and 4 were 0 kg/m^3 , 0.91 kg/m^3 , 1.82 kg/m^3 and 2.73 kg/m^3 respectively, table 1a. Mix 1 does not contain any polypropylene, 0 kg/m^3 , was used as the control mix whereas mix 2, 0.91 kg/m^3 , was recommended by the manufacturer. Four mixes were also used to study the effect of the steel fibers on the engineering properties of concrete, mixes 1, 2, 3 and 4 contain 0 kg/m^3 , 10 kg/m^3 , 15 kg/m^3 and 20 kg/m^3 of steel fibres respectively, table 1b. Mix 1, 0 kg/m^3 , was the control mix and mix 2, 10 kg/m^3 , was also recommended by the manufacturer.

Table 1a also shows the mix proportion of the concrete mix used for the test described in the previous paragraph, it contains 400 kg/m^3 of cement and w/c ratio of 0.5 was used. The same mix properties, table 1b, were used to study the effect of steel fiber on the shear capacity of reinforced concrete beams. The steel fiber content was 20 kg/m^3 , identical to mix 4 which it was used to study the mechanical properties of concrete.

Table 1a: Polypropylene mix proportions for mixes 1 (control), 2, 3 and 4.

Ingredients	Source	Unit	Weight m ³
OP Cement	UCC	Kg	400
Water	MUN	Ltr	200
20mm aggregate	RAK	Kg	700
10mm aggregate	RAK	Kg	330
5mm aggregate	RAK	Kg	515
Sand – Fine	RAK	Kg	295
Polypropylene (Mixes 1,2,3 and 4)	Dramix	Kg	0.0, 0.91, 1.82 or 2.73

Table 1b: Steel fibers mix proportions for mixes 1 (Control), 2, 3 and 4.

Ingredients	Source	Unit	Weight m ³
OP Cement	UCC	kg	400
Water	MUN	Ltr	200
20mm aggregate	RAK	Kg	700
10mm aggregate	RAK	Kg	330
5mm aggregate	RAK	Kg	515
Sand – F	RAK	Kg	295
Steel Fibers (Mixes 1,2,3 and 4)	Dramix	Kg	0, 10, 15 and 20

Polypropylene fibers are available as monofilament 12 mm in length for concrete use and 6 mm in length for plaster and mortar. The former was used in this investigation. The advantages of using polypropylene in concrete are;

- a. Inhibits the formation of small cracks which can occur through plastic shrinkage, premature drying and early thermal changes
- b. replaces anti cracks wire mesh
- c. improves concrete durability and finishes characteristics
- d. increase impact and abrasion resistance
- e. impervious to alkali attack

Typical content of polypropylene is shown in table 1c.

Table 1c: Typical content of Polypropylene.

Typical content of polypropylene	
Specific gravity	0.91 gm/cm ³
Alkali, sulphat and chloride content	nil
Fiber thickness	18 micron

Steel fibers concrete is more durable than plain concrete made from the same mix design. The fiber within the alkaline environment of the concrete are protected from corrosion, the aspect ratio (length/diameter = L/d) of the fibers has a major effect on workability. Three different steel fibers with different L/d ratios of 45, 65 and 80 are available for basic requirements, quality control and minimum performance respectively. The latter was used for this investigation. These glued fibers offer the best in term of ductility, energy absorption and crack control.

4. Manufacture Of The Specimens:

The samples used to study the effect of polypropylene and steel fibers on the mechanical properties of concrete were cast in steel moulds; whereas the beams used to investigate the effect of steel fiber on the shear capacity of concrete were cast in timber moulds. The samples were vibrated externally using a table vibrator. After trowelling the concrete surface smooth, the beams were stored under polythene sheets. All the specimens were un moulded after one day and then cured for 28 days in water tank at 20°C and 100% relative humidity. The tests were carried out within 1 hour of removal of the samples from the conditioning tanks.

5. Apparatus And Test Procedure:

The samples for investigating the mechanical properties of polypropylene concrete and steel fiber reinforced concrete were tested in a 3000kN controlled compression machine. The rate of loading was $0.3\text{N}/(\text{mm}^2.\text{s})^{[7]}$, all the specimens were seated directly on the rigid bed of the machine.

For testing the shear capacity of steel fibres concrete beams, the load was applied to the 8 beams using a 150kN hydromantic jack, giving a constant moment zone of 440mm and shear span to effective depth ratio of 3.59. The supports were 50mm from the ends of the beams, giving anchorage of 3.5 and 8.5 times the bar diameter for poor and good anchorage respectively, as shown in figure 1.

6. Test results and discussion

The static tests conducted to investigate the effect of both polypropylene and steel fiber on the mechanical properties of plain concrete are shown in table 2, whereas the results of the effect of steel fiber, only, on the shear capacity of reinforced concrete beams are shown in tables 3.

Table 2: The effect of Polypropylene and steel fiber on the mechanical properties of concrete.

Properties of Concrete	Steel Fiber				Micro Polypropylene Fiber			
	Mix 1 – Control (0kg/m ³)	Mix 2 (10kg/m ³)	Mix 3 (15kg/m ³)	Mix 4 (20kg/m ³)	Mix 1 – Control (0kg/m ³)	Mix 2 (0.91kg/m ³)	Mix 3 (1.82kg/m ³)	Mix 4 (2.73kg/m ³)
Cube Compressive Strength (N/mm²)	52.58	58.04	58.32	58.34	44.78	43.70	39.03	37.57
Cylinder Compressive Strength (N/mm²)	21.44	27.52	29.32	29.67	30.98	26.74	24.98	21.97
Indirect Tensile Test (N/mm²)	6.64	5.69	5.46	5.43	5.13	4.79	4.43	3.77
Modulus of Rupture (N/mm²)	12.35	12.75	12.80	13.25	6.32	6.29	6.21	6.17
Modulus of Elasticity (N/mm²)	42.27	39.63	35.23	22.12	42.27	28.82	27.57	26.42
Water Absorption (g)	0.11	0.10	0.10	0.11	97	101	104	107
Slump (mm)	140	140	145	140	115	100	70	50

Table 3: The Effect of Steel Fiber on the static strength, shear capacity, of reinforced concrete beam.

Beam Code No.	Ultimate Load (kN)	% Gain	Design Load (kN)	Static failure occurred at
Control Beam 1	34.0		30.00	Shear Span
Steel Fiber Beam 1	40.0	17.65%	30.00	Shear Span
Control Beam 2	40.0		30.00	Shear Span
Steel Fiber Beam 2	46.0	15.00%	30.00	Shear Span
Control Beam 3	56.0		30.00	Shear Span
Steel Fiber Beam 3	69.5	24.11%	30.00	Shear Span
Control Beam 4	60.0		30.00	Shear Span
Steel Fiber Beam 4	71.0	18.33%	30.00	Shear Span

Beam 1: Beam with no links and poor anchorage

Beam 2: Beam with no link and good anchorage

Beam 3: Beam with links and poor anchorage

Beam 4: Beam with links and good anchorage

6-1 The results of the first part of the research which is the effect of different percentage of polypropylene and steel fibres on the engineering properties of concrete is discussed below

6-1-1 Compressive Strength

The effect of different percentage of polypropylene and steel fibres on the compressive strength of concrete are shown in table 2. The cube compressive strength of concrete with polypropylene decreases as the content (kg/m^3) of polypropylene increases. The drop in compressive strength for mixes 2, 3 and 4 are approximately 2.4%, 14.7% and 19.9% compared with the control mix 1 - 0kg/m^3 of polypropylene fibres. The increase of polypropylene, PP, content slightly increases the absorption of water and therefore affects the process of hydration and in turn reduces the compressive strength of concrete. Water and polypropylene, are grossly dissimilar in polarity and the lack of affinity for one another^[8] which means that the permeability of water through PP is low. The polarity mismatch also applies between PP and concrete, and this is likely to cause poor wetting at the interface between the fibres and concrete, so it is quite clear that the dissimilar in polarity and the lack of affinity between water and polypropylene, PP, and also between the latter and concrete affected the hydration process of concrete and therefore affect the strength of concrete. This is also explain the small variation of water absorption and the drop in slump values of mixes 2, 3 and 4 of the polypropylene, PP, concrete, table 2.

On the other hand the addition of steel fibres to the concrete increases the cube compressive strength. The increase of the content of steel fibres in mixes 2, 3 and 4 showed an increase in the compressive strength of concrete cube of 10.38%, 10.92% and 10.95 compared to the control mix, mix 1, respectively. The increase in compressive strength of steel fibres, SF, concrete cube could be due to the increase in interlocking between concrete and the steel fibres. Further-more the orientation of the steel fibres impede the propagation of the crack due to the load and therefore delay the ultimate failure. It can also be seen that the addition of steel fibres to concrete has no effect on water absorption and the slump value, table 2.

It is quite clear that the increase in the compressive strength of steel fibres concrete does not depend on the amount of steel fibres added to the mixes, therefore increasing the steel fibres volume from 0 kg/m^3 in mix 2 to 15 kg/m^3 in mix 3 and 20 kg/m^3 in mix 4 gave almost the same compressive strength values. The reasons are that there is no increase in the energy which extend the crack and debond the fibres in the matrix.

The results of the cylinder compressive strength for both polypropylene, PP, and the steel fibres, SF, show similar trend to the cube compressive strength, table 2.

6.1.2 Tensile Strength of Concrete

The effect of the polypropylene on the tensile strength of concrete is shown in table 2, the addition of the polypropylene, PP, decreases both the indirect tensile strength and flexural strength, i.e. modulus of rupture. The reduction in the indirect tensile strength for mixes 2, 3 and 4 are approximately 6.6%, 13.65% and 26.51% compared with the control, mix 1 - 0kg/m³, respectively. On the other hand the decrease in the flexure strength, MoR, for mixes 2, 3 and 4 are approximately 0.5%, 1.7% and 2.4% compared with the control mix 1 - 0kg/m³, respectively.

It is clear that the polypropylene, PP, reduces the absorption of water and therefore affects the process of hydration; this creates non uniform stress distribution which leads to crack. Once the latter has formed, it can propagate quickly through the section of the specimen and in turn reduces the tensile strength of concrete.

Table 2 clearly shows that the increase in the steel fibres content decreases the indirect tensile strength but increases the Flexure strength, MoR. The reduction in the indirect tensile strength for mixes 2, 3 and 4 are 14.20%, 17.67% and 18.05% compared with the control, mix 1 - 0kg/m³, respectively. On the other hand the increase in the flexure strength, MoR, for the same mixes are 3.24%, 3.64% and 7.29% compared with the control mix 1 - 0kg/m³, respectively. This could be due to the fact that the steel fibres, which are randomly distributed, do not aligned in the direction of the tensile stress. Therefore, it is quite clear that the steel fibres have less effect on the flexural strength than the indirect tensile strength.

The limited tests reported by Nguyen Van Chanh^[9] and also by Jeflery R Roesler at al^[10] showed that the steel fibres and the polypropylene do little to enhance the static compressive and tensile strength of concrete. Information regarding the other engineering properties discussed in this research was not reported by them.

6.1.3 Modulus of Elasticity, MoE:

Table 2 shows that the moduli of elasticity of both the polypropylene concrete and steel fibres concrete decreases as the PP and SF were added to the mixes. The MoE of PP and SF decreases as the content of the PP and SF fibres were increase in the mixes. Table 2 shows that the addition of 0.91 kg/m³, 1.82 kg/m³ and 2.73 kg/m³ of polypropylene fibres to the concrete mix 1, 2 and 3 decreases the MoE by 31.82%, 34.78% and 37.5 % compared to the control mix, mix 1 - 0kg/m³ of polypropylene, respectively. On the other hand the addition of the steel fibres to mix 2, 3 and 4 also causes reduction in the modulus of elasticity of 6.25%, 16.65% and 47.67% compared with the control, mix 1 - 0kg/m³, respectively, figure 10. The reduction in the moduli of elasticity of both the polypropylene concrete and the steel fibres concrete could be due to the fact that the moduli of elasticity, MoE, of the polypropylene and the steel fibres are low compare to the MoE of the corresponding properties of the matrix [11].

6.1.4 Mode of Failure

Figure 1 shows the mode failure of SF concrete cylinder tested in compression. The control specimens failed suddenly and there were no visible cracks before failure. This type of explosive failure was not observed on specimen cast from the SF mixes 2, 3 and 4. Therefore, Steel fibres in concrete, SF, resist the sudden explosion. It is clear that the samples fail longitudinally and explosive failure did not take place. Similar failure mode was reported in cylinder tested in tension. The polypropylene, PP, has no effect on the failure mode of the concrete cylinders tested in compression or tension. The latter fail in the same way as the control cylinder. It is clear that the steel fibres increase the resistant of the concrete to sudden failure.

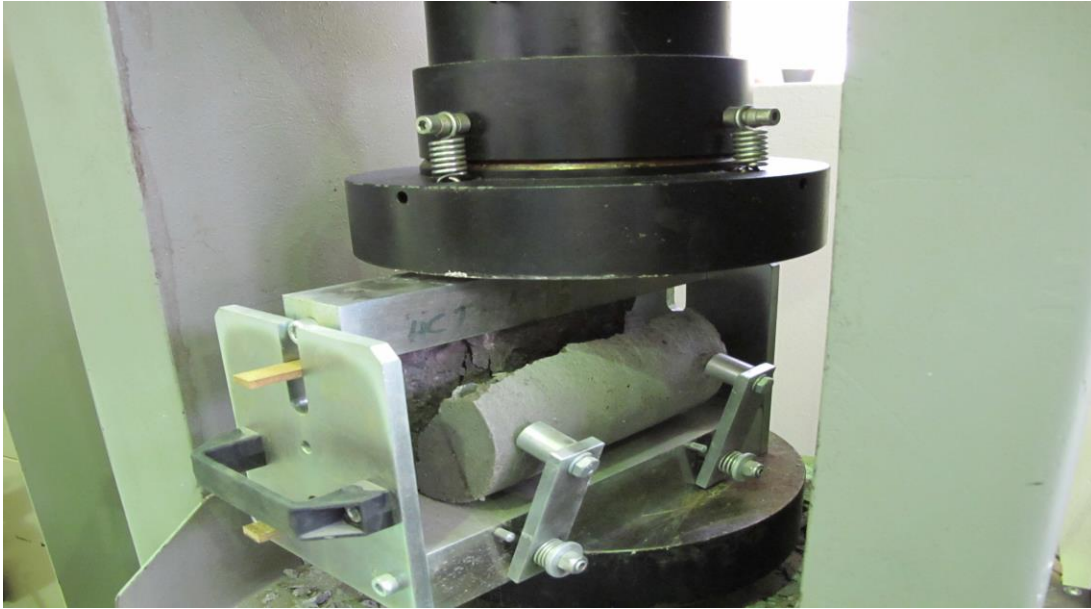


Fig. 1: Failure mode of Steel Fibres concrete cylinder tested in compression.

6.2 – This section discusses the result of the second part of the research: The effect of steel fibres on the shear capacity of reinforced concrete beams

Since the effect of polypropylene on concrete was not significant on the engineering properties of concrete, this part of the research concentrate on the effect of steel fibres on the shear capacity of reinforced concrete beams. Based on the findings from the experimental investigation, the following conclusions are drawn from table 3;

- 1- The steel fibres increase the shear capacity of reinforced concrete beams with or without shear links. The percentage increases in strength depends on whether the beam is singly reinforced or doubly reinforced.
- 2- Table 3 also show that in the beams without links, the shear strength increased by 17.65 % and 15 % for beam 1 and beam 2 respectively, compared to the corresponding control specimens.
- 3- The steel fibres reinforced concrete beam, B2, and its control beam show an increase in the ultimate shear capacity than the steel fibres beam, B1, and it's control beam. Therefore the good end detailing increases the ultimate shear strength of the concrete beams.
- 4- Steel fibres reinforced concrete beams B3 and B4 show an increase in strength of 24.11% and 18.33% respectively compared to the corresponding control specimens.
- 5- In general the increase in the ultimate shear capacity of the steel fibres reinforced concrete beams is due to the fact that the shear forces were resisted by the increase in the compressive strength of concrete due to the presence of the steel fibres, as well as the transverse and longitudinal reinforcement. Kuang J.S et al^[12] and Bilal et al^[13] agreed that the addition of steel fibres in concrete leads to a substantial enhancement of shear carrying capacity of RC beam. The parameters tested by both researchers were different to the parameter tested in this research.
- 6 - The increase of the shear capacity of the SF reinforced concrete beams is also due to the fact that ^[14];
 - The random distribution of fibres throughout the volume of concrete at much closer space has led to distributed cracking with reduced crack size.
 - The fibres also increase the first-crack tensile strength and ultimate tensile strength of the concrete.
 - The shear friction strength is increased by resistance to pull-out and by fibres bridging cracks.

7 - The deflection of the Steel fibres reinforced concrete beams, figure 2, was less than that of the corresponding normal concrete beams prior to cracking. The SF beams showed slightly higher deflection at failure as they failed at higher loads. Thus, SF reinforced beams have adequate deformation capacity in spite of their failure mode. The increasing deflection rate just prior to failure for all the beams suggested that the steel was beginning to yield and the beams, therefore, had reached their maximum load capacity.

8 - It can be noted from figure 3 that the steel fibres reinforced concrete beams SF1 and SF2 show less steep shear crack direction compared to the shear crack of their conventional reinforced concrete beams C1 and C2. The presence of the steel fibres, compression reinforcement and links in the steel fibres reinforced concrete beams SF3 and SF4 increases the compressive strength of the concrete and therefore causes steep shear crack direction compared to their control beams C3 and C4 respectively. Typical shear failure was recorded for all the beams; therefore it is clear that the increase in the shear capacity of the SF reinforced concrete beams did not alter or prevent the diagonal tension failure of the beams.

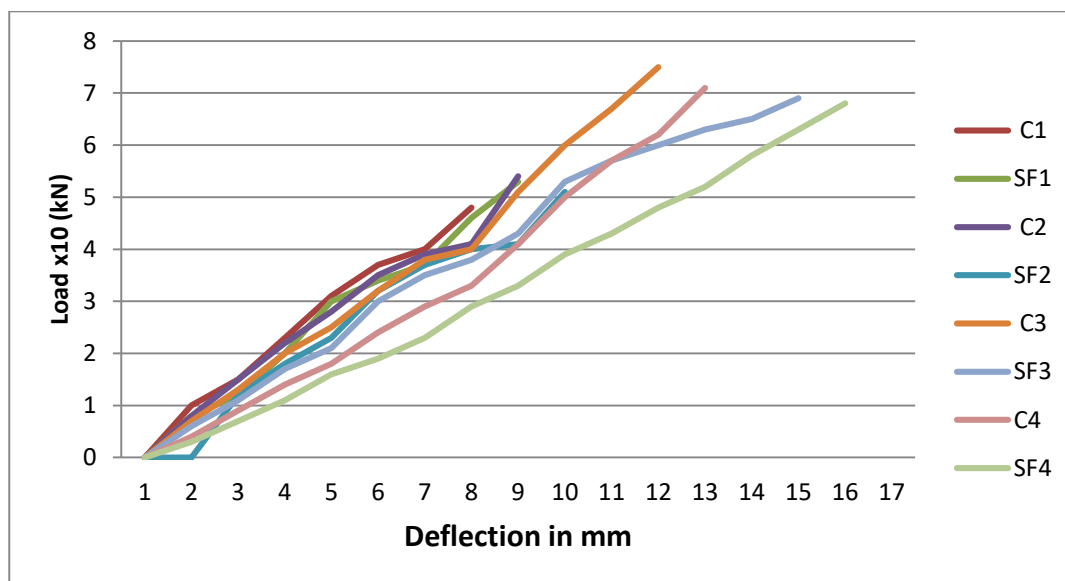


Fig. 2: Deflection of Steel Fibres Beams SF1, SF2, SF3, SF4 and their corresponding control against Load.

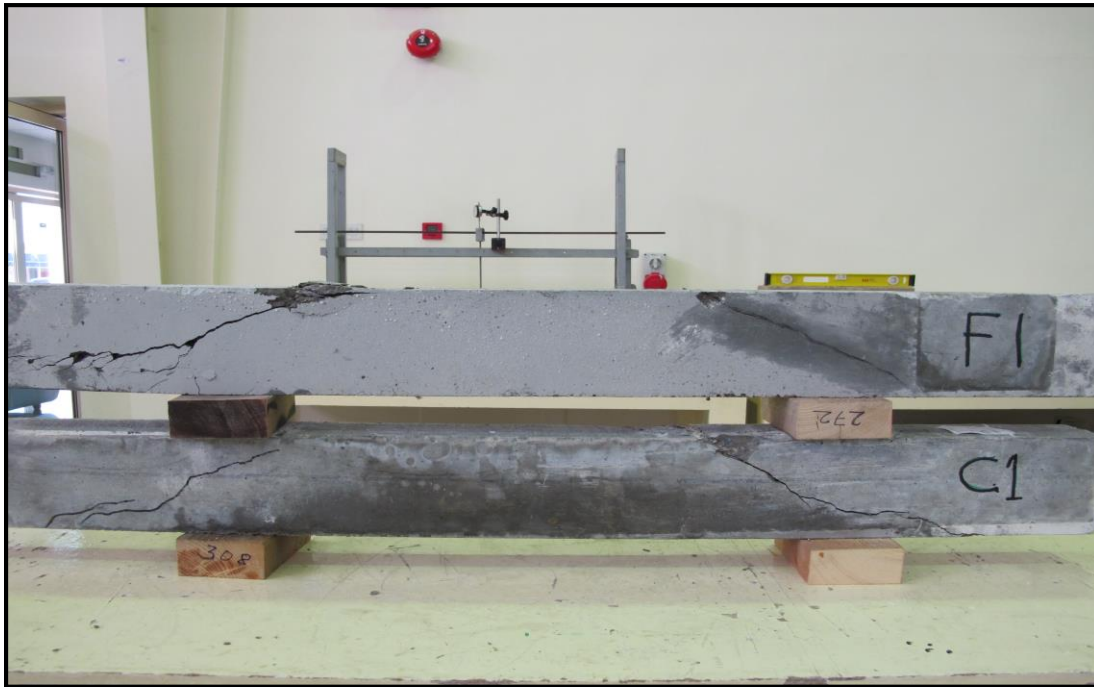


Fig. 3: Shear failure mode under static load, for steel fibres, SF1, and control beam, C1.

7. Conclusions

Based on the analysis of the test in the previous sections, the following conclusions are deduced;

1 - The addition of polypropylene slightly increases the water absorption but decreases the mechanical properties of concrete such as; cube compressive strength, cylinder compressive strength, indirect tensile strength, modulus of rupture, modulus of elasticity and the slump values. This could be due to the dissimilar in polarity and the lack of affinity between the PP and water. These lower the permeability of water and therefore affect the process of hydration which it leads to reduction in ultimate strength. On the other hand the steel fibres increase the cube compressive strength, cylinders compressive strength and the modulus of rupture of concrete but it decreases the indirect tensile strength and modulus of elasticity of concrete. This is due to the increase in interlocking between concrete and steel fibres and also the orientation of the steel fibres impede the propagation of the crack due to the load and therefore delay the ultimate failure. The water absorption and slump values of the SF concrete remain unchanged.

2 - The steel fibres in reinforced concrete beams enhance the shear capacity of singly or doubly reinforced concrete beams with or without good anchorage. The increase in shear strength, due to steel fibres, in beams with links was slightly greater at 18% regardless of the quality of the end anchorage of the main reinforcement compared to the control beams. The increase of the shear capacity of the SF concrete beams is due to the fact that the random distribution of fibres throughout the volume of concrete at much closer space has led to distributed cracking with reduced crack size. The fibres also increase the first-crack tensile strength and ultimate tensile strength of the concrete. The increase in shear capacity of the SF concrete beams can also be explained by the fact that the shear friction strength is increased by resistance to pull-out and by fibres bridging cracks. All the control and SF reinforced concrete beams failed in shear.

3 - The Steel fibres reinforced concrete beams tested in shear showed lower deflection than their corresponding normal concrete beams prior to cracking. Thus, SF reinforced beams have adequate deformation capacity in spite of their failure mode. The increase in deflection rate just prior to failure for all the beams suggested that the steel was beginning to yield and the beams, therefore, had reached their maximum load capacity.

4 – A combination of vertical stirrup and randomly distributed steel fibres reinforcement in the matrix enhance the diagonal tension capacity of the concrete beams.

Acknowledgment

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