# Fresh and Hardened Properties of Engineered Geopolymer Composite with MgO

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**Abstract-** In this paper, the early results of an ongoing investigation on self-healing engineered geopolymer composites (EGC) are presented. The EGC was developed using powder based alkali activators and MgO was added as self-healing agent. Two types of source materials were used to produce EGC. One EGC mix had slag and class C fly ash as source materials and termed as binary mix. The other one had slag, class C fly ash and class F fly ash and termed as ternary mix. Setting time, slump flow, fresh density and rheology were measured as fresh properties of the developed geopolymer composites. As hardened properties, compressive and direct tensile strengths were evaluated. It was observed that addition of MgO delayed the setting time of both the EGC mixes. The rheology of the developed geopolymer mixes complemented the hardened properties of the mixes. It was found that binary geopolymer mix exhibited superior performance as compared to its ternary counterpart due to presence of class C fly ash only that ensured higher amount of CaO. It was also observed that EGC, developed in the present study, experienced strength values (both compressive and direct tensile) that are comparable to the values of the previous studies even with the addition of MgO. Moreover, strain hardening characteristic was observed for both EGC mixes under direct tension test. Hence, it is evident that the initial outcomes of the experimental investigation are quite promising and exhibit the importance of conducting further comprehensive studies in order to develop design guidelines for EGC with self-healing capability.

Keywords: Engineered geopolymer composites; powder based activators; MgO; Binary mix; Ternary mix; Strain hardening

#### 1. Introduction

It is beyond doubt that construction industry is facing enormous challenge to meet the ever increasing demand for Portland cement [1]. Moreover, production of Portland cement requires considerable amount of energy consumption due to clinker calcinations and grinding of raw materials [2-4]. The high energy consumption of cement manufacturing process also results in high emission of  $CO_2$ . Each ton of Portland cement clinker production emits around 0.9 to 1 ton of  $CO_2$  [5, 6]. Consequently, it has been estimated that  $CO_2$  emission from cement production constitutes about 8% of global  $CO_2$  emission [7]. Hence, it is obvious that cement manufacturing process has severe impact on environment. In this context, significant attempts have already been initiated to reduce the environmental impact of cement production through reduction of clinker content. The clinker content is reduced by replacing it with supplementary cementitious materials (SCMs) like ground granulated blast furnace slag (GGBS), natural pozzolans, fly ash (FA) etc. [2, 8, 9]. Such utilization of SCMs definitely enhances the sustainability of cement and concrete industry. However, considering the rapid growth of cement industries due to fast urbanization, particularly in developing parts of the world, development of clinker free concrete is of immense potential for reduction of  $CO_2$  emission from construction industry.

Geopolymer (GP) is a clinker free binder and is considered as a promising alternative to Portland cement since it can achieve comparable strength and better durability [1, 10-12]. Application GP can reduce up to 80% of CO<sub>2</sub> emission as compared to Portland cement by totally replacing clinker with FA and/or GGBS [13, 14]. GP is amorphous alkali aluminosilicate in which tetrahedral silicate and aluminate are linked through covalent bonds in a three-dimensional structure. The geopolymerization process involves quick reaction of silica (Si) and alumina (Al) under alkaline condition. Therefore, raw materials of GP must consist of two components i.e., reactive aluminosilicate solids and alkaline activating agent. In fly ash/slag based geopolymer, FA and/or GBBS supply free  $[SiO_4]^-$  and  $[AIO_4]^-$  in the solution in the presence of alkaline activating agent i.e., alkali hydroxide/alkali silicate. Common solution based alkaline reagents used to produce geopolymer

are sodium hydroxide, potassium hydroxide, sodium silicate and potassium silicate [1, 15]. The  $[SiO_4]^-$  and  $[AIO_4]^-$  units form three-dimensional polymeric chain of Si–O–Al–O bonds by sharing oxygen atom as shown in Equations 1 and 2 [1, 16, 17]. This aluminosilicate gel acts as a binder in geopolymer based cementitious composites.

$$(Si_2O_5Al_2O_2)_n + H_2O + OH^- \to Si(OH)_4 + Al(OH)^{4-}$$
(1)

$$Si(OH)_{4} + Al(OH)^{4-} \rightarrow \begin{pmatrix} | & | \\ Si-O - Al-O \\ | & | \end{pmatrix}_{n} + 4H_{2}O$$

$$\tag{2}$$

Alkali activator solution plays a very important role to initiate the geopolymerization process [18]. In activation process of geopolymerization reactions, the most dominant parameters are the nature and concentration of activators [18, 19]. Komljenovic et al. [19] utilized five different alkali activator solutions and found activation potential of the activators in the order of Na<sub>2</sub>SiO<sub>3</sub>>Ca(OH)<sub>2</sub>>NaOH+Na<sub>2</sub>CO<sub>3</sub>>KOH based on compressive strength of the developed geopolymers. In addition, the properties of solution based geopolymers have been found to be depended on  $SiO_2/Al_2O_3$  ratio, curing temperature, molarity of alkali activators solution, amount of water addition, R<sub>2</sub>O/Al<sub>2</sub>O<sub>3</sub> ratio and SiO<sub>2</sub>/ R<sub>2</sub>O ratio (where, R =  $Na^+$  or  $K^+$ ) [1, 18, 20-24]. It is evident that a significant number of studies are available in literature on development of solution based geopolymer. However, utilization of solution based activators possesses several limitations to be executed in field conditions to produce large volume of geopolymers [25, 26]. High temperature curing is usually required to activate the polymerization reaction in FA based geopolymer with activator solutions [27, 28]. In addition, application of large volume of solution based activators in the field is quite difficult due to high alkalinity and corrosive nature [25, 29]. Some recent studies show that relative small quantities of alkali activators in powder form (as compared to solution based activators) can activate source materials at ambient temperature [25, 30, 31]. Therefore, difficulties associated with field application and production cost of geopolymer can be lessened by using dry mix of source materials and activators. However, study on dry mix based geopolymers is still quite limited and further research works should be conducted for developing specification and design guidelines specifically for geopolymers with powder based activators.

Researchers at Ryerson University have played significant role in developing ultra-high performance concrete (UHPC), self-consolidating concrete (SSC), lightweight self-consolidating concrete (LWSCC), and engineered cementitious composites (ECC) mixes [32-35]. A comprehensive study has undertaken recently by the same research group to develop geopolymer based smart engineered cementitious composite (smart EGC: smart Engineered Geopolymer Composite) with self-healing and/or self-sensing properties using dry mix technique. Self-healing EGC has been proposed to develop using MgO type expansive agent since MgO has already been successfully incorporated to develop self-healing ECC [36]. In this article, the initial results of the experimental investigation on EGC with MgO are presented. The geopolymers have been produced using two different combinations of FA-Class C (FA-C), slag and FA-Class F (FA-F) using activators in powder form. Commercially available Polyvinyl alcohol (PVA) fiber has been used to produce EGC. Several relevant fresh and hardened properties of geopolymer mortar/EGC with MgO have been evaluated to demonstrate the effect of addition of MgO within geopolymer. The authors believe that findings of the study would contribute significantly for conducting further research to develop EGC with self-healing ability.

#### 2. Materials and Methodology

The binary and ternary geopolymer mixtures were developed by mixing FA-C+slag and FA-C+slag+FA-F, respectively as reactive aluminosilicate solids with fine silica sand (SS), water and polycarboxylate based high range water reducing admixture (HRWRA/SP). Commercially available Polyvinyl alcohol (PVA) fiber with 8 mm in length and 40 µm in diameter was used to produce engineered geopolymer composite. The PVA fiber surface was coated with 2% oil by weight to reduce fiber/matrix frictional bond. A particular combination of Na<sub>2</sub>SiO<sub>3</sub> and Ca(OH)<sub>2</sub> was used as alkali activator in powder form based on finding of a previous study [25]. In addition, lightly burnt MgO was prepared from pure Magnesite (MgCO<sub>3</sub>) following the procedure described in [37] to be used as self-healing agent with geopolymers. The details of prepared EGC mixes with MgO are provided in Table 1. Geopolymer mortar was prepared following the same mix proportions except the addition of PVA fibers.

As fresh properties, setting time and rheology of geoplolymer mortar with MgO and slump flow and fresh density of EGC with MgO were evaluated. Setting time was measured following ASTM C807 standard [38]. Brookfield DV3T Rheometer was used to determine the rheology of geopolymer mortar. The slump flow and fresh density of EGC were measured as per ASTM C1611 [39] and ASTM C138 [40] standards, respectively. Compressive and direct tension (Dogbone) tests of EGC were determined following ASTM C109 [41] and ASTM E8 [42] procedures, respectively. The hardened density of EGC was evaluated following conventional procedure. Figure 1 shows samples/setup of different tests conducted in the study.

Table 1: Details	of binary	and	ternary	mixes
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Mix ID	Types of Source Materials (SM)	SM proportion (Parts by	MgO*	Activator*	Silica Sand*	SP*	PVA**	Water/SM (w/S)
BM1F+MgO	FA-C+slag	1	0.05	0.1	0.3	0.02	0.02	0.35
TM1F +MgO	FA-C+slag+FA-F	1	0.05	0.1	0.3	0.02	0.02	0.35

\*weight proportion with respect to SM; \*\*volume proportion with respect to total volume of the mix



Fig. 1: Samples prepared for different tests.

# 3. Results and Discussion

# 3.1. Setting Time and Densities of the Mixes

The initial and final setting times of geopolymer mortar are provided in Table 2. For each mix composition, two sets of samples were tested and the deviation in observed values in almost all cases was found to be around  $\pm 2\%$ . It is evident that geopolymer with binary mix (BM1F+MgO) experienced relatively early setting as compared to its ternary counterpart (TM1F+MgO). The initial and final setting times of BM1F mix were observed to be about 36 and 40 minutes less than that of TM1F mix, respectively. The presence of FA-F in ternary mix was responsible for the delay in setting times of TM1F since it increased the SiO<sub>2</sub>/Al<sub>2</sub>O<sub>3</sub> ratio [1]. In addition, delay in setting time through addition of MgO was also evident from

the experimental results since previous studies observed initial setting time of up to 120 minutes for typical geopolymer mixes [1, 43]. Such behaviour was due to the retarding effect of MgO within geopolymer matrix. Table 2 also shows fresh and hardened densities of EGC mixes. Both mixes showed almost identical fresh and hardened density values.

Sample name	Setting time		Fresh density	Hardened density	
	Initial (min)	Final (min)	$(kg/m^3)$	(kg/m <sup>3</sup> )	
BM1F+MgO	164	230	2207	2017	
TM1F+MgO	200	270	2209	2013	

Table 2: Summary of setting time, fresh and hardened density

#### 3.2. Rheology and Slump Flow

Material rheology characterizes its consistency, how it will spread over a surface as well as deformation characteristics [44, 45]. When it comes to cement by-products or as such (pastes, mortars or concrete), rheology seems to affect their microstructural, mechanical and durability features, together with their fluidity [45]. During rheological testing, flocs formed due to hydration process are broken down by running a viscometer with a consistent shear rate. As for this study, rheology of the two geopolymer mortar mixes were observed using Brookfield DV3T Rheometer and were subjected to a rotor speed of 20 rpm. Figure 2 shows variation of viscosity of two mixes with respect to time. For each mix composition, three sets of samples were tested and the variation in observed values in almost all cases was found to be 3 to 5%. Initially, viscosity of the binary mix decreased from a value of 9100 cP (Centipoise) to 7150 cP in 15 minutes. This can be accounted to still workable mix. As the time progressed, more water was used for hydration. Therefore, the mix started to set and loosed its consistency and thus, the viscosity increased from 7150 cP to around 12500 cP. Torque values were also observed to show similar trend of variation within the range of 36% to 27% that was within limit (10% to 100%). As for ternary mix, the variation over time was observed to be less fluctuating. However, the value, similar to the binary mix, first reduced to 5850 cP within 15-20 minutes and then increased up to 8191 cP as the mix started to set. The torque values were observed to decrease from 15% to around 11% and then rose up to 16%. Both the trend showed that the mixes exhibited high viscosity value as it started to set and loose consistency. However, the binary mix experienced relative high viscosity over time due to presence of high amount of CaO that eventually resulted in better hydration. The strength test results discussed in Section 2.3 also complement this hypothesis.



Fig. 2: Variation of viscosity with time.

The slump flow test was conducted using the traditional slump cone. The slump cone was completely filled without any vibration and the time required to reach a spread of 500 mm (from the moment of lifting up the cone) by the EGC mixes was measured. This is called the  $T_{50}$  measurement and typically varies between 2 and 10 seconds for self-consolidating concrete.

The  $T_{50}$  measurement for BM1F+MgO was 7 second and for TM1F +MgO was 8 sec. Hence, both EGC mixes exhibited very good flow satisfying criteria for self-consolidating concrete.

#### 3.3. Compressive and Direct Tensile Strengths

Figure 3(a) shows the compressive strengths of the developed EGC mixes at different ages of 7, 14 and 28 days. For each mix composition and each day, a set of five samples were cast and the standard deviation of the obtained results were in the range of  $\pm 1$  to 2 MPa. The highest 28 day compressive strength of 41.5 MPa was achieved by the binary EGC (BM1F-MgO) mix. The 28 day compressive strength of the ternary mix (TM1F-MgO) was 38.4 MPa. The relative lower strength of ternary mix was due to comparatively lower activation of FA-C [25]. From previous studies it has been found that compressive strength of fly ash based geopolymer with solution based activator ranged from 30 to 80 MPa [1, 46] depending on water to geopolymer solid (source materials) ratio and curing temperature. On the other hand, compressive strengths of EGC were found to be in the range of 17.4 to 54.6 MPa in previous research studies conducted by Ohno and Li [47] and Nematollahi et al. [48]. Both studies used fly ash as source materials and combinations of NaOH and Na<sub>2</sub>SiO<sub>3</sub> as activators. The maximum compressive strength of the geopolymer mix [46] and EGC [48] was achieved by the water to source materials (w/S) ratio of 0.18 and 0.20, respectively. Both the w/S ratios were significantly less than the ratio used in the current study. Hence, there is scope for further improvement in compressive strength of EGC with MgO by reducing the water content. Moreover, addition of MgO increased the water demand and caused dilution of source material that resulted in further decrease in strength attainment. However, MgO contributes to attainment of strength at later age through formation of M-S-H which was outside the scope of the present article. Considering all the above mentioned factors, it could be concluded that EGC with MgO exhibited satisfactory performance in compressive strength tests.



Fig. 3: (a) Compressive strength of EGC mixes; (b) Stress vc strain relationship of EGC mixes obtained from direct tension tests.

Figure 3(b) shows the tensile strength vs strain graphs of both the EGC mixes with MgO at 28 days obtained from direct tension test. Similar to compressive strength, the binary mix exhibited superior performance in the direct tension test. The maximum tensile strength of 3.8 and 3.3 MPa was achieved by the binary and ternary mix, respectively. Previous studies on EGC by Ohno and Li [47] and Nematollahi et al. [48] found uni-axial tensile strength ranged from 2.9 to 3.4 MPa and 3.9 to 5.2 MPa, respectively. Therefore, tensile strengths obtained in the current study by EGC with dry mix activators and MgO are comparable to previous studies and further enhancement can be expected resulting from the late pozzolanic reactions between Mg(OH)<sub>2</sub> and Si. In addition, both EGC mixes with MgO experienced strain hardening, the prerequisite to develop engineered geopolymer composites.

# 4. Conclusions

The study presents early results of an ongoing comprehensive investigation on engineered geopolymer composites (EGC) with powder based/dry mix activators and MgO. Magnesium Oxide was added to develop self-healing EGC. Two EGC mixes were produced using combination of FA-C+slag and FA-C+slag+FA-F termed as binary and ternary mix, respectively. The following conclusions can be drawn from the present study;

- The binary mix exhibited relatively quicker initial and final setting times as compared to ternary mix. In addition, both EGC mixes experienced comparatively slower setting times in comparison with conventional geopolymer mixes due to addition of MgO.
- The binary mix experienced higher viscosity with time than that of ternary mix resulting from better hydration caused by higher amount of CaO. Torque values of both the EGC mixes were found to be within the limit.
- The binary mix achieved higher compressive strength at 28 days in comparison with ternary mix. However, both the EGC mixes experienced compressive strengths that are comparable to the strength of geopolymer mixes of previous studies having similar water to source material ratio. Moreover, the long term pozzolanic reaction of MgO addition has not been considered in this study which could further improve the strength of the EGC mixes.
- Similar to compressive strength, the binary mix achieved higher tensile strength than that of ternary mix. The tensile strengths of both the EGC mixes are comparable to the previous studies despite addition of MgO which has retarding effect on strength attainment.
- Both EGC mixes exhibited satisfactory strain hardening characteristics.

It is, therefore, obvious that initial outcomes of the experimental investigation are quite promising and exhibit potential of developing EGC as sustainable construction materials. However, further studies with more sample size will be required to develop design specifications for EGC.

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