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Effect of Specimen Size and Shape on the Mechanical Properties of Recycled Aggregate Concrete

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Abstract - The use of green concrete is increasing worldwide. The growing global demand for utilizing recycled materials in concrete mixes to enhance sustainability makes it important to study the strength properties and behavior of recycled aggregate concrete (RAC). While the impact of specimen shape and size on the compressive and flexural strength of normal concrete is well-documented, their effects on concrete mixes containing recycled aggregate and ceramic waste are not yet defined. This paper studies the effect of specimen shape and size on the strength properties of RAC by conducting compression tests on cubes and cylinders and performing four-point flexural tests on beams. Locally available recycled coarse aggregate, ceramic fine aggregate, and ceramic waste powder were used to develop the recycled aggregate concrete. Compression tests were conducted on standard cubes, standard cylinders, and half-scale cylinders. Flexural tests were performed on beams sized 150 mm×150 mm×460 mm and 75 mm×75 mm×230 mm. Based on the experimental results, it was found that the specimen shape and size significantly affect the strength properties of RAC, and the conversion factors differ from those of normal concrete with the same strength. Additionally, a preliminary relationship between the compressive strength and flexural strength of RAC is suggested.

Keywords: Green concrete, Ceramic waste material, Size effect, Sustainability, Strength properties.

1 Introduction

In recent years, the construction industry has been blooming, and subsequently, the demand for concrete has rocketed. As a result, concerns regarding the impact of concrete on the environment and natural resources started increasing. The main constituent of concrete is the ordinary Portland cement (OPC). The production of OPC is responsible for 5-7% of the CO2 global emissions [1]. Moreover, the demand for natural sand and aggregate reached 32-50 billion tonnes annually [2], a rate that exceeds the natural renewal capacity and can result in sand and aggregate supply shortage by mid-century [3]. On the other hand, construction and demolition (C&D) waste produced annually increases the burden on the environment [4]. For the mentioned reasons, the research to use recycled construction material and reduce the cement in concrete mixes to produce greener concrete has arisen globally over the past years [5-7]. Studies investigated the use of alternative cementitious materials such as fly ash and ground granulated blast furnace slag (GGBFS) [8], biochar [9], or the use of recycled waste materials such as waste glass [10], plastic [11], and recycled aggregate from construction waste [12]. However, advancements in structural engineering research continue [13-16] with reinforced concrete remaining the primary material.

Recycled aggregate (RA) is produced through crushing, cleaning, and screening of C&D waste, and then used again in fresh concrete mixes [17]. Many authors investigated the use of recycled aggregate concrete (RAC) in Ordinary Portland Cement Concrete (OPCC). Most researchers reported a decrease in concrete quality as the natural coarse aggregate replacement ratio with RA increased. It was reported that the use of RA resulted in a lower workability of fresh concrete [18] and 5%-15% more water was needed to achieve the same workability as the 100% natural aggregate concrete (NAC) which is attributed to the increased water absorption of recycled aggregate [19]. Moreover, the utilization of RA usually results in a lower compressive strength, in particular, if the replacement ratio of natural aggregate with RA is 100%, the drop in compressive strength can reach up to 20-25% [20]. Moreover, RA has a negative effect on the elastic modulus of concrete [21]. However, studies were carried out to investigate enhancing the bond

between the RA and the cement mix by treating the RA surface using different techniques [17], [22], and acceptable [23] and high-performance mixes [24] were developed using RA.

Compressive and flexural strength are the most important mechanical properties of concrete and are crucial in the designing of concrete structures. Compressive strength can be obtained by testing standard cylinders and cubes under compressive forces until failure [25]. On the other hand, flexural strength is assessed by testing of beam specimen subject to a four-point or three-point bending test [26]. At times, an estimate of the flexural strength can be obtained by using the established relationship between the compressive strength and the flexural strength. The most common form of this relation is:

$$f_{ct,f}' = a f_c'^b \tag{1}$$

where $f_{\text{ct.f}}$ is the flexural tensile strength in MPa, $f_{c'}$ is the compressive strength in MPa, and "a" and "b" are factors and different codes have different values for 'a' and 'b'. For instance, the AS 3600:2018 suggests that a = 0.6 and $b = \frac{1}{2}$ [27] while ACI 318-19 suggests that a = 0.62 and $b = \frac{1}{2}$ [28]. However, further investigation is needed to check if this relationship is affected by the change in the type of aggregate or cement.

However, the strength properties of concrete are not unique and they are affected by the change in some factors including the size and the shape of the sample, generally, cubes show higher strength than cylinders, and smaller samples show higher strength than larger samples [29-32]. In the AS 3600:2018 for instance, a conversion factor of 0.8 was given to convert from standard cube compressive strength to standard cylinder compressive strength [27]. However, this is suggested for ordinary concrete reinforced and may not be applied to every type of cement or aggregate as changing the mix materials results in a change in the way concrete behaves. In addition, although some of the standards such BS 8500-2:2023 [25] allow the use of cubes of different sizes and ASTM C39 [33] allows the use of cylinders of different sizes, none of them mention the use of a size factor to convert from one sample size to the other. Moreover, the Australian standards [27] recognize the existence of size effect by limiting the combination of the results from the two cylinder sizes that it uses, however, it does not specify a conversion factor between the two sizes. This suggests the need for more research to be done on the specimen shape and size effect on the concrete mechanical properties, in particular, the more novel concrete mixes such as RAC.

Various studies have been done to investigate the effect of sample size and shape on Recycled Aggregate Concrete (RAC). Some literature reports a cube-to-cylinder conversion factor of 0.78–0.93 for RAC, with no size effect observed [34]. In another study [35], the cube-to-cylinder conversion factor for RAC, using 200 cylinders and cubes, was found to be 0.7. The size effect was also examined for RAC in cubes [36] and cylinders [37], both of which reported a negative size effect, indicating greater strength with larger samples. However, none of these studies tested the size effect on flexural strength, nor did they utilize 100% recycled aggregate or ceramic waste materials in their concrete mixes.

This paper aims to examine the effect of specimen size and shape on the mechanical properties (compressive and flexural strength) of recycled aggregate concrete while utilizing 100% recycled coarse aggregate and ceramic waste materials.

2 Experimental Setup

2.1 Concrete Mix

The concrete mix used in this study was developed by Batikha et al to investigate the use of recycled coarse aggregate and ceramic waste materials and their effect on the strength properties of concrete [38]. In this mix, 100% of the natural coarse aggregate was replaced with recycled aggregate, 20% of the natural fine aggregate was replaced with ceramic fine aggregate, and 20% of Portland cement was replaced with ceramic waste powder. The mix was designed to achieve a targeted characteristic cylindrical compressive strength of 40 MPa and a mean cylindrical compressive strength of 40 MPa [38]. The same mix was adopted with a slight increase in the water-to-cement ratio from 0.4 to 0.5. The mix design is shown in Table 1.

Material	Quantity / m ³		Quantity / m ³	
	Liter (L) / Kilogram (kg)	Material	Liter (L) / Kilogram (kg)	
OPC	320 kg	Natural fine aggregate	796 kg	
Ceramic	00.1-	Ceramic fine	100 1	

aggregate

Water

Superplasticizer

80 kg

827 kg

Table 1: Concrete mix design.

2.2 Materials

The cement used in this study is Type I Ordinary Portland Cement class 42.5 N. Black sand and ceramic fine aggregate with a maximum particle size of less than 5 mm were used as fine aggregate. Moreover, Recycled Coarse Aggregate (RCA) with a maximum particle size of 14 mm was obtained from Beea'h Recycling factory. The water absorption for recycled aggregate is 4.8%. The ceramic waste powder (CWP) used in this study passed the 125-micrometer sieve. The superplasticizer used in this study is poly-carboxylate ether with the commercial name of PC400 supplied by PAC technology. Figure 1 shows pictures of RCA and CWP.

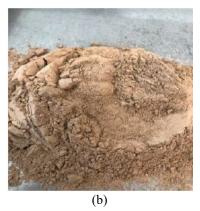


Waste

Powder Recycled

Coarse

Aggregate



199 kg

160 L

5 L

Fig. 1: (a) Recycled Coarse Aggregate (RCA); (b) Ceramic Waste Powder (CWP).

2.3 Sample Type and Size

Samples of various shapes and sizes were tested, including standard-size cubes of $150 \times 150 \times 150$ mm and standard size cylinders measuring 150×300 mm, and half-scale samples measuring 75×150 mm. For each size, nine samples were cast. Three samples of each size were subject to compressive strength testing on the 7th, 14th, and 28th days. Beam sizes of $150 \times 150 \times 460$ mm and half scale size of $75 \times 75 \times 230$ mm, with three samples cast for each size, were tested for flexural strength using the four-point bending test on the 28th day. Table 2 provides a summary of the specimen, test specifications, and age of testing.

Specimen shape and No. of Age **Test** specimens size Days 150×150×150mm Cubes 7, 14, Compressive 75×150 mm Cylinders Strength 28 150×300 mm 9 Cylinders

3

3

28

150×150×460 mm

Beams

75×75×230 mm

Beams

Table 2: Type of specimen and age at test.

3 Results and Discussion

3.1 Effect of sample shape on compressive strength

Flexural

Strength

The effect of specimen shape on compressive strength is evident in the results (see Table 3), as the 150×300 mm cylinders showed lower compressive strength compared to the 150×150×150 mm cubes. An average conversion factor of approximately 0.5 was determined to convert from the standard cube size to the standard cylinder size across all curing ages. This is lower than the provisions of AS 3600 [27] where the average conversion factor is around 0.8. This difference may be attributed to the lower content of Ordinary Portland Cement (OPC) in the mix, as it was partially replaced by ceramic waste powder. Figure 2(a) shows development of compressive strength of cubes with regards to curing age.

Sample shape/type	Sample size	Mean strength at 28 days MPa	Standard deviation	Coefficient of variation
Cubes	150×150×150	61.6	3.82	6.2
Cylinders	150×300	30	2.1	6.9
	75×150	39.9	1.48	3.71
Beams	150×150×460	4	0.75	18.92
	75×75×230	6.4	0.21	3.23

Table 3: Experimental results.

3.2 Effect of size on compressive strength

The small-scale cylinders (75×150 mm), which are half the size of the standard 150×300 mm cylinders, showed higher strength than the standard-size cylinders, as shown in Table 3. The scale factor to convert from half-scale cylinders to standard-size cylinders is 0.78, indicating the effect of size on compressive strength. This size factor is not constant across the curing age (in days) as can be seen in Figure 2(b). Figure 3(a) illustrates the strength development of the different shapes and sizes of concrete samples.

3.3 Effect of Size on Flexural Strength

The impact of size on flexural strength was evident in this experiment. The smaller $75 \times 75 \times 230$ mm beams, which are half the size of the $150 \times 150 \times 460$ mm beams, displayed a higher flexural strength of 6.4 MPa compared to 4 MPa.

The size conversion factor from small to large beams is 0.625. Flexural strength results from study [29] can be utilized to further analyze the size effect. Despite being cured for 56 days, the $100 \times 100 \times 500$ mm beam used in that study exhibited lower strength (4.6 MPa) than the $75 \times 75 \times 230$ mm beams (6.4 MPa). Additionally, the $100 \times 100 \times 500$ mm beam had a smaller width and depth, and it showed higher flexural strength than the large beams in this study (4 MPa). Size factors for converting from the $100 \times 100 \times 500$ mm beam tested in Batikha et al study to the small and large beams tested in this study can be 1.36 and 0.87, respectively. However, these size factors might not be extremely accurate as they do not account for differences in W/C ratios and curing ages. Figure 3(b) illustrates the size effect on flexural strength (including the beam from study [29]).

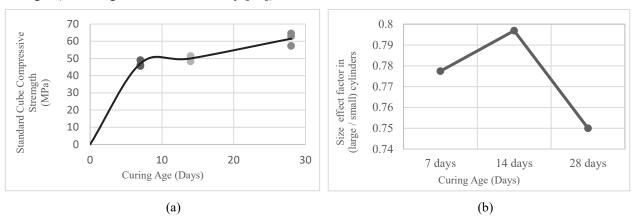


Fig. 2: (a) Development of compressive strength of cubes with regards to curing age; (b) Size effect with regards to curing age.

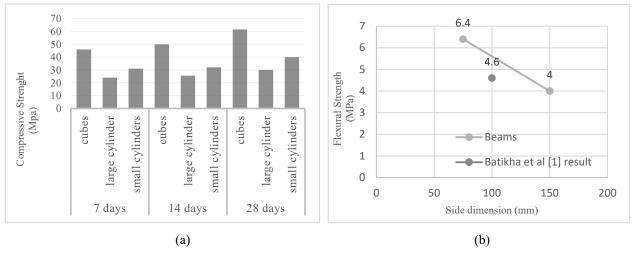


Fig. 3: (a) Compressive strength with regards to various specimen shapes and sizes; (b) Flexural Strength with respect to beam side dimensions.

3.4 Relationship between compressive and flexural strength

The results of the standards cylinder and large beams obtained in this study suggest the a, and b factors in Eq. (1) to be 0.74 and 0.5 respectively, making the final Eq. (2):

$$f_{ct.f}' = 0.74 f_c'^{\frac{1}{2}} \tag{2}$$

However, this is a preliminary equation and further testing on a greater number of samples needs to be done to confirm the accuracy of this equation.

4 Conclusion

This study investigated the effect of the specimen size and shape on the compressive and flexural strength of recycled aggregate concrete. Samples of various configurations were cast: nine standard cubes, nine standard and half-scale cylinders, and three beams of large size and half-scale were prepared and water-cured. Three cubes and cylinders of each size were tested for compressive strength after 7,14, and 28 days, while beams were tested for flexural strength after 28 days. The following conclusions were drawn:

- The average cylinder/cube ratio was found to be 0.5.
- The effect of the sample size was observed on half and full-scale cylinders. A size factor of 0.78 was developed to convert from small to large cylinders.
- The size effect was present in the flexural strength of the beams as well. A size factor of 0.625 was found to convert from small to large beams.
- Lastly, a preliminary relationship between compressive cylinder strength and flexural strength of concrete large beams was suggested, though more testing needs to be done to confirm the viability of the equation.

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